

## **Staff Report and Comments**

**W.D. Beaty House**

**2400 Kendrick Drive**

**Charlotte, NC**

**Application for COA HLC395**

### **Exhibits presented to and considered by the Commission:**

**Exhibit A** – Project Description

1. New single family house at 2405 Kendrick Drive

**Exhibit B** – Map

**Exhibit C** – Project Plans

### **Based upon the information presented in the application, staff offers the following suggested findings of fact:**

The HLC has acknowledged the need to alter or add to a historic property to meet continuing or new uses while retaining the property's historic character.

1. The proposed construction is on the subdivided parcel of the Beaty House property. The new house is a one story, single family ranch style house similar in character, setting and scale to adjacent original houses from the 1950s.
2. The proposed project meets the HLC Standard 1. A property shall be used for its historic purpose or be placed in a new use that requires minimal change to the defining characteristics of the building and its site and environment.
3. The proposed project meets the HLC Standard 2. Distinctive materials, features, finishes and construction techniques or examples of craftsmanship that characterize a property will be preserved.

**Staff suggests that the Commission approve the application as presented.**

## **THE HLC STANDARDS**

**Rehabilitation is defined as the act or process of making possible a compatible use for a property through repair, alterations, and additions while preserving those portions or features which convey its historical, cultural, or architectural values.**

1. A property shall be used for its historic purpose or be placed in a new use that requires minimal change to the defining characteristics of the building and its site and environment.
2. The historic character of a property shall be retained and preserved. The removal of historic materials or alteration of features and spaces that characterize a property shall be avoided.
3. Each property shall be recognized as a physical record of its time, place, and use. Changes that create a false sense of historical development, such as adding conjectural features or architectural elements from other buildings, shall not be undertaken.
4. Most properties change over time; those changes that have acquired historic significance in their own right shall be retained and preserved.
5. Distinctive features, finishes, and construction techniques or examples of craftsmanship that characterize a historic property shall be preserved.
6. Deteriorated historic features shall be repaired rather than replaced. Where the severity of deterioration requires replacement of a distinctive feature, the new feature shall match the old in design, color, texture, and other visual qualities and, where possible, materials. Replacement of missing features shall be substantiated by documentary, physical, or pictorial evidence.
7. Chemical or physical treatments, such as sandblasting, that cause damage to historic materials shall not be used. The surface cleaning of structures, if appropriate, shall be undertaken using the gentlest means possible.
8. Significant archeological resources affected by a project shall be protected and preserved. If such resources must be disturbed, mitigation measures shall be undertaken.
9. New additions, alterations, or related new construction shall not destroy historic materials that characterize the property. The new work shall be differentiated from the old and shall be compatible with the massing, size, scale, and architectural features to protect the historic integrity of the property and its environment.
10. Alterations, new additions, and adjacent or related new construction shall be undertaken in such a manner that if removed in the future, the essential form and integrity of the historic property and its environment would be unimpaired.



N 87°44'40" E 320.06' (TOTAL)

230.06'

6'-TIE

90.00'

13.5' SY(ZONING)

27' FS(ZONING)

27' FS(ZONING)

LOT 1  
BLOCK 4  
MB 6 PG 949  
PARCEL ID  
#055-294-07

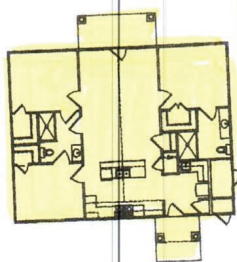


N.C.

OF THE UNIFIED  
ID DOES NOT  
NEVER, ANY FURTHER

T  
VLY  
ION

CP-CORNER  
ITS IN UTILITY POLE



**LOT 2**  
28798 SF  
0.661 AC

CONCRETE DRIVE

CONCRETE DRIVE

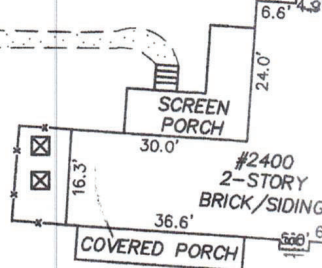
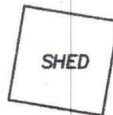
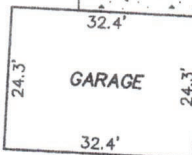
**LOT 1**  
66586 SF  
1.529 AC

N 02°15'20" W 320.00'  
5' SY(ZONING)

S 02°15'20" E 319.96'  
5' SY(ZONING)

5' SY(ZONING)

CONCRETE WALK



40' RY(ZONING)

40' RY(ZONING)

S 87°43'03" W 90.00'

S 87°28'06" W 240.86'-TIE  
MB 1844 PG 485

S 87°43'03" W 151.04'

L2-TIE  
CNF

APPARENT GAP BETWEEN

LOT 1  
BLOCK E  
MB 1844 PG 485



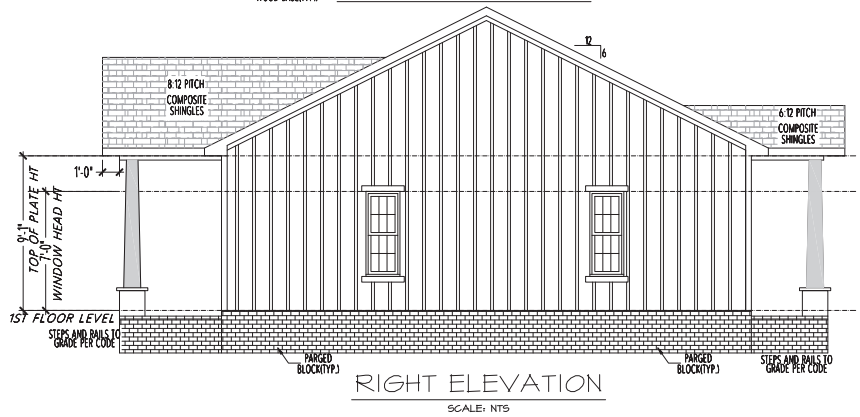




- ELEVATION NOTES**
1. GRADE ELEVATIONS SHOWN DO NOT NECESSARILY REFER TO THIS OR ANY OTHER LOT. THEY ARE FOR DIAGRAMMATIC PURPOSES ONLY AND MAY VARY. BUILDER IS RESPONSIBLE FOR ADAPTING THIS PLAN TO SUIT THE EXISTING TOPOGRAPHY OF THE SITE.
  2. WINDOW GRILLS SHOWN ARE TO SCHEMATICALLY EXPRESS DESIGN INTENT. ACTUAL STD. GRILLS MAY VARY PER MANUFACTURER OR CUSTOM GRILLS MAY BE REQUIRED. ANY VARIATIONS FROM THAT SHOWN MUST BE APPROVED BY BUILDER.
  3. ROOF VENTILATION TO BE DETERMINED BY BUILDER AS PER CODE. DOWNSPOUTS NOT SHOWN FOR CLARITY.
  4. CONTRACTOR TO VERIFY LOCATIONS W/ OWNER.
  5. ALL CONSTRUCTION TO COMPLY WITH WITH N.C.R.C. 2018 RESIDENTIAL CODE.

Residential Designer will not be responsible for human error after construction begins and assumes no liability of the Residential Designer for any modifications made to these plans by others. These plans are not to be reproduced without the written consent of the Residential Designer.

THE CONTRACTOR ASSUMES FULL RESPONSIBILITY FOR THE CORRECT INSTALLATION OF ALL EXTERIOR FINISHES AND WEATHERPROOFING.



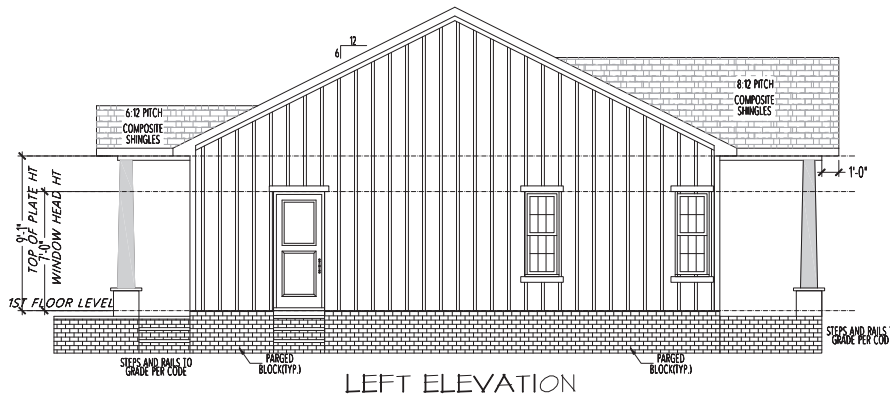
MEMBER  
**AIBD**  
AMERICAN INSTITUTE OF BUILDING DESIGNERS

RESIDENTIAL DESIGNER  
Charlotte, NC  
www.aibd.com

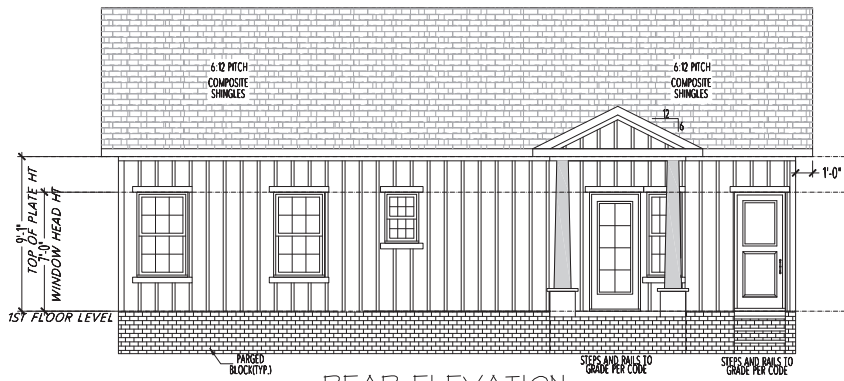
**STAY I LLC**  
COBLE RESIDENCE  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28214

DATE:	8.4.23
SCALE:	NTS
PROJECT:	3.0

3.0



LEFT ELEVATION



REAR ELEVATION

SCALE: NTS

MEMBER  
**AIBD**  
AMERICAN INSTITUTE OF  
BUILDING DESIGNERS

DESIGNER  
**STAY LLC**  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28214  
PH: 704.366.1111  
WWW.STAYLLC.COM

**STAY LLC**  
COBLE RESIDENCE  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28214

DATE:	8.4.23
PROJECT:	COBLE RESIDENCE
DRAWN BY:	MAVIN
CHECKED BY:	CAJ

SHEET NO.  
**4.0**



1. DESIGN LOADS:

- 1.1 Design loads are all dead loads plus:
A. Main floor live loads (kitchen level) 40 PSF
B. All other floors 40 PSF
C. Balconies 60 PSF
D. Decks 50 PSF
E. Suspended Garages 50 PSF
and 2000 Pound Point Load at any location
F. Attic floor live loading with the following:
i. Areas accessible by permanent stairs 30 PSF
ii. With Storage 40 PSF
iii. Without Storage 10 PSF
G. Roof live load 30 PSF
H. Wind load 115 MPH (3 Second Gust)
I. Conform with Seismic Design Criteria for Zone C.
J. Snow load 20 PSF
1.2 All designs are in accordance with the 2018 North Carolina Residential Building Code, designed using ASD 2301.2.1 for all wood and steel structural elements and LRFD 2301.2.2 for all concrete structural elements.

2. FOOTINGS AND FOUNDATIONS:

- 2.1 Soil bearing capacity assumed as 2000 PSF unless noted otherwise or as determined by standard penetrometer test.
2.2 All continuous wall footings for one or two-story houses are 10" thick x 20" wide. Reinforcing in footings should be two (2) #4 bars if not noted on the plans. Reinforcing not required by Code, unless footings are on disturbed soil or compacted fill.
2.3 All interior piers are 8"x16" CMU up to a maximum height of 32". All piers over 32" high must be filled with Type S mortar. Maximum height for 8"x16" filled pier is 6'-4". Piers larger than 8"x16" are noted on the plans or as required by height. Pier cap blocks should be 8" of solid masonry.
2.4 Footings for 8"x16" piers are 20"x30"x16" unless noted otherwise. Reinforcing to be as noted on plans.
2.5 Concrete shall have a compressive strength of 3000 PSI in 28 days unless noted otherwise. No concrete shall be poured in temperatures below 40° Fahrenheit unless heat is provided during curing for two days. The bottom of all footings must be a minimum of 12" below grade.
2.6 All rebar splices shall be a minimum of 2'-0" unless otherwise noted.
2.7 Any special foundations for structures shall be designed by a Licensed Professional Engineer upon receiving soil capacity specifications for all soil considered to affect the structure.
2.8 Chimney footing loads are shown on the structural design drawings. Masonry or Inconcrete chimney footings must be a minimum of 12" thick with 12" projection on all sides.
2.9 Foundation walls back-filled with soil and supporting structural framing shall be constructed as shown on detail sheet.
2.10 Special retaining wall designs to be shown on detail sheet.

NOTE: ALL POINT LOADS FROM ROOF BRACES, JACK STUDS, AND BEAM SUPPORTS - WHETHER WOOD OR STEEL - CANNOT BEAR ON SITING ALONE. BLOCKING EQUAL TO OR BETTER THAN THE SPECIFIED STUDS OR COLUMN PROVIDED FOR POINT LOAD SUPPORT MUST BE CARRIED THROUGH ALL CONSTRUCTION TO THE FOUNDATION.

3. FRAMING CONSTRUCTION - OTHER THAN ROOF:

- 3.1 Crawlpace girders and bands as noted on plans. Maximum clear span to be 4'-8" (6'-0" o/c spacing of piers) unless noted otherwise.
To avoid most cracking in finished hardwood floors over any girders, use the following procedure:
A. Nailing Patterns
i. All floor joists must be toe-nailed to support girders with a minimum of 3-8d nails at each end from each side. Larger nails will split and reduce the toe-nail ineffective. No end-cauling through the girder or band is permitted except for temporary construction purposes.
ii. If dropped girders are used, end-lap all joints 12" minimum and side-wall each with a minimum of 3-16d nails at each end of each joint. Ledger strips should be nailed with 3-16d nails at each joint end, with nails spaced 3" apart.
iii. Nail multiple-member bulk-up girders with three rows of 16d nails staggered at 32" o/c, 2" down from the top, 2" up from the bottom, and at mid-depth. Use 3-16d nails at each end of each piece in the joints through the members making up the multiple-girder. This nailing pattern will insure a tight floor from outside of house to outside so that when the framing shrinks during the first heating season, the shrinkage will be uniformly distributed over the entire floor.
If the girder nailing pattern is omitted, then the shrinkage will accumulate over the girders and an objectionable crack will develop in the finished hardwood floor over the girder line.
B. At all girders where the joints change direction, install bridging at 6" o/c for a minimum of six joint spacings beyond any joint direction change. This will insure shrinkage distribution over the floor and not let it accumulate at the girder.
C. There must be wood blocking through-bolted to the steel beam with joint toe-nailed and attached to the beam with metal hangers under any hardwood floors that pass over a steel beam supporting floor joists.

3. FRAMING CONSTRUCTION - OTHER THAN ROOF (CONTINUED):

- 3.2 All framing lumber must be Spruce Pine
Ft #2 unless noted otherwise.
3.3 Steel beams must have 5-2x4 jack studs under each end support unless noted otherwise on the structural plans. All studs must be nailed together with two (2) vertical rows of 16d nails at 8" o/c, unless noted otherwise.
3.4 LVL beams must have 3-2x4 jack studs under each end support unless noted otherwise on the structural plans. All studs must be nailed together with two (2) vertical rows of 16d nails at 8" o/c, unless noted otherwise.
3.5 Masonry lintels:
A. For spans up to 6 ft. Use 3/8"x2 1/2"x16" steel angles.
B. For spans from 6 ft to 10 ft. Use 5/8"x3 1/2"x16" steel angles.
C. For spans from 9 ft to 18 ft. Use a pair of 9 gauge wire splices 12" minimum and extend wires 12" minimum into jacks. Temporarily support steel angle before laying masonry. Shoring may be removed seven days following the installation of masonry.
D. When structural steel beams with bottom plates are used to support masonry, the bottom plate must extend the full length of the steel beam. This provides support to the ends of the plate by bearing on the adjacent masonry jacks. The beam should be temporarily shored prior to laying the masonry. The shoring may be removed five days after laying the masonry.
3.6 All masonry or stone veneer over lower roofs must have a structural steel angle lag bolted to the adjacent wall studs to prevent sliding of the veneer. A minimum of a triple rafter must be installed below masonry chimneys. This set veneer attachments provided by the contractor may supersede this specification. Please verify the alternative attachment procedure with the Engineer of Record.
3.7 All rafter braces must have 2 studs from the wall top plate through all floors solid to the foundation or supporting beam below. No braces shall be attached to the top wall plate without studs directly under them.
3.8 Where non-bearing parallel partitions fall between floor joists, 2x4 ladders @ 16" o/c must be placed perpendicular to the joists to support the plywood decking or double joist installed directly below wall.
3.9 All wood I-joints must be braced in accordance with the manufacturer's directions plus any details shown on the plans. Load bearing partitions, jacks, beams and column supports must be solidly blocked through the floor as the joists and plywood may not be able to carry the concentrated point loads. All point loads must be carried to the foundations with blocking and/or beams. (NOTE: All beams and double joists, etc, have been shown for a load bearing purpose. Placement of the load carrying members shown in the plans in locations other than under the structural element they are intended to carry is the responsibility of the contractor. Exact beam locations are not to be scaled from the framing plans.)
3.10 All two-story open rooms with full height openings must be braced to resist pressure resulting from 115 MPH design fastest-mile wind speed or as prescribed for specified wind zones per ASCE 7-98. Any special wall reinforcing shall be shown on the plans provided. Two-story open rooms must be balloon-framed with 2x6 @ 16" o/c as a minimum (no exceptions).
3.11 Stud walls to be listed below unless otherwise noted on the structural plans:
A. Interior One & Two Story Walls (with intermediate floors)
i. Load bearing 2x4 @ 16" o/c
ii. Non-load bearing 2x4 @ 16" o/c
B. Interior Three Story Walls
i. Load bearing (2nd & 3rd Floor) 2x4 @ 16" o/c
ii. Load bearing (1st Floor) 2x4 @ 12" o/c or 2x4 @ 16" o/c
iii. Non-load bearing 2x4 @ 16" o/c
C. Basement Walls
i. Load bearing 2x4 @ 12" o/c
ii. Non-load bearing 2x4 @ 16" o/c
D. Exterior Walls
Esterior walls for three stories shall be 2x6 @ 16" o/c with 1/4"x4"x8" OSB sheathing or C-DS plywood over entire exterior.
3.12 Headers shall be as shown on the plans.
3.13 When ceiling joists are parallel to an exterior wall and rafters bear on the exterior stud wall's top plate, tie the rafters near the top plate to the ceiling joists with 6 long 2x6 runners at 4' o/c across the top of the ceiling joist.
3.14 At all bay windows, each panel shall be nailed to each adjacent panel with 5-16d nails tied together with metal strapping nailed at four locations between floors with a minimum of 2-16d nails in each panel at each step. This will help prevent vertical cracking in the panel joints due to horizontal oscillation of the panels.
3.15 At all stairs, every stud at each stringer must be nailed to each stringer with a minimum of 2-16d nails. This will help prevent cracking between the wallboard and the top of the base molding due to vertical oscillation of the stair stringers.
3.16 Steel pipe columns must be in contact with the supported member and continue solid to the supporting masonry or concrete foundation. No intermediate wood blocking should be used as it will crack. Pipe columns are to be welded to the bottom flange of all steel beams with a continuous fillet weld. Steel plates should be welded to the top of pipe columns with two holes to allow for a minimum of two 3/8" diameter screws into all wood beams.

4. FOUNDATION WALLS

- 4.1 All full height foundation walls are shown on structural detail sheet.
4.2 All masonry or concrete basement wall construction must be inspected by the County Building Official, Architect, or Engineer for compliance with structural specifications.
4.3 Where full-height foundation or basement walls run parallel to floor framing, blocking must be provided between joists at 3'-0" o/c for not less than six joint spacings out from wall.
4.4 Details of any earth retaining structures not attached to the house structure will be shown on separate details. (These walls may be designed only after grade conditions are known.)

5. ROOF CONSTRUCTION

- 5.1 Rafters shall be 2x6 SPP #2 @ 16" o/c for standard weight shingles except as noted. They are to be cut into hips, ridges, etc., unless noted as over-balls.
5.2 Collar ties shall be 2x6 @ 48" o/c at all ridges unless noted otherwise and located a minimum 3" below the ridge. Collar ties may be closer to ridge if alternate bracing provided. Vaulted ceilings require special collar tie details or structural ridge beam. See plans as required.
5.3 A minimum of three collar ties shall be used at all ridges even if two ties may be put on one set of rafters.
5.4 All hips and ridges are a size larger than the rafters framing into them unless noted otherwise.
5.5 All hogs on ceiling joists or rafters are 8" long 2x6 hog troughs unless noted otherwise. Rafters may be spliced over hogs.
5.6 Gable end framing must be braced parallel to ridges with a minimum of 2x6 diagonal braces @ 6' o/c along the gable wall to the interior ceiling joist. Braces are to bear on 2x6 hogs and top plate at approximately mid-height of gable wall. Braces shall be at approximately a 45° angle. Other bracing may be used if it meets the Engineer's approval.
5.7 Carry braces to partitions or beams below. Never brace rafter hogs to 2x6 hogs on ceiling joists, unless shown on plans.
5.8 Ceiling joists when erected parallel to rafters must be sistered to rafters and nailed with 3-16d nails at each rafter. If a knee-wall is used and ceiling joists cannot touch rafters, then rafters must be braced to the ceiling joist with 2x4 diagonal rafter ties spaced @ 48" o/c. Reverse collar ties may be used behind kneewalls.
5.9 Roof Flus Legend:
A. Arrow location of roof brace at rafter level.
B. Arrow away from brace point indicates direction of roof brace to partition, beam or other brace point below.
C. Arrow into brace point indicates a vertical or almost vertical roof brace to partition, beam or other brace point below.
D. All roof braces are 2x4 1" nailed with 16d nails @ 9" o/c vertically from top to bottom. All braces longer than 10' must be braced horizontally in two directions at mid-height or be increased to 2x6s.
E. Maximum spacing of roof braces to be as follows:
i. For 2-2x6 hog 6'-0" o/c
ii. For 2-2x8 hog 7'-6" o/c

6. WALL BRACING PER R. 602.10

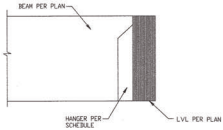
This structure has been analyzed by the professional engineer of record for lateral loading. It has been designed using continuous (C) OSB sheathing fastened to the exterior wall framing with 8d nails at 6" on center and 12" on center in the field, to meet and exceed the intent of the 2018 North Carolina Residential Building Code. Where braced wall lines require additional reinforcing, engineered walls sections and hold downs have been provided.

All R00H hold downs are to be Simpson LSTA15 or MSTA15 vertical straps fastened to a minimum of a two stud pocket and the floor band.

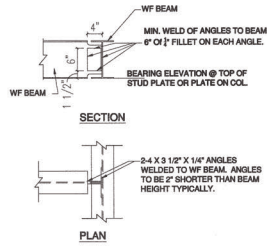
EMF- Engineered Moment Frame



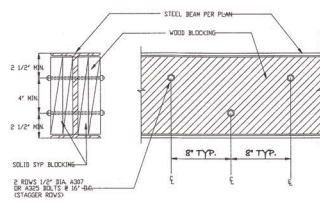
Structural Engineer: ROBERT S. SEEFORD, License No. 10000, Exp. 12/31/2024. PROJECT NAME AND ADDRESS: COBLE RESIDENCE, 2405 KENDRICK DRIVE, CHARLOTTE, NC 28214. DRAWING TITLE: GENERAL NOTES. DATE: SEPT. 6, 2023. SHEET NO: SGN.



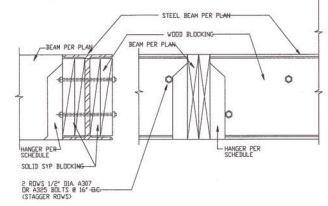
1 BEAM TO LVL CONNECTION DETAIL  
SCALE=1/8"=1'-0"



2 STEEL TO STEEL CONNECTION DETAIL  
SCALE=1/8"=1'-0" N/A



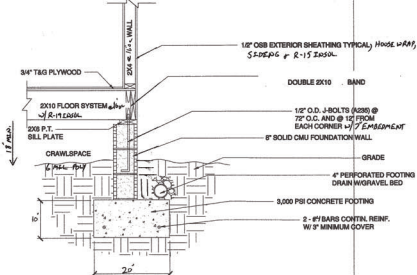
3 STEEL BEAM WEB BLOCKING DETAIL  
SCALE=1/8"=1'-0" N/A



4 BEAM TO STEEL BEAM CONNECTION DETAIL  
SCALE=1/8"=1'-0" N/A

TYPICAL HANGERS FOR JOISTS AND BEAMS	
MEMBER	HANGER
2x8	L2080
2x10	L2820
2x12	L3580
2x16	H4080
2x18	H4500
2x20	H5000
2x22	H5500
2x24	H6000
2x26	H6500
2x28	H7000
2x30	H7500
2x32	H8000
2x34	H8500
2x36	H9000
2x38	H9500
2x40	H10000
2x42	H10500
2x44	H11000
2x46	H11500
2x48	H12000
2x50	H12500
2x52	H13000
2x54	H13500
2x56	H14000
2x58	H14500
2x60	H15000
2x62	H15500
2x64	H16000
2x66	H16500
2x68	H17000
2x70	H17500
2x72	H18000
2x74	H18500
2x76	H19000
2x78	H19500
2x80	H20000
2x82	H20500
2x84	H21000
2x86	H21500
2x88	H22000
2x90	H22500
2x92	H23000
2x94	H23500
2x96	H24000
2x98	H24500
2x100	H25000

5 HANGER SCHEDULE  
SCALE=1/8"=1'-0"



6 CRAWLSPACE FOUNDATION DETAIL  
SCALE=1/8"=1'-0"

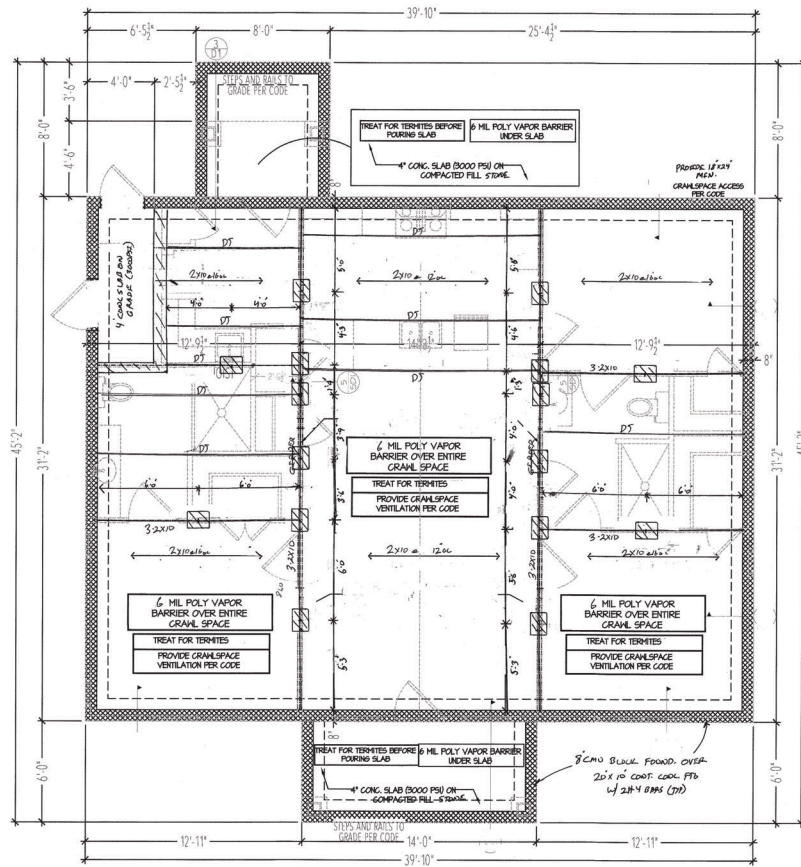
STRUCTURAL ENGINEER  
Professional Seal  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28224

PROJECT NAME AND ADDRESS  
COBLE RESIDENCE  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28224

DRAWING TITLE  
STANDARD DETAILS

DATE  
SEPT. 6, 2022

SHEET NO.  
SD1



**CRAWLSPACE FOUNDATION**  
SCALE: NTS



Sustainable Engineering & Efficient Designs, P.L.L.C.  
3445 Overland Avenue  
Charlotte, NC 28202  
SEAL FOR STRUCTURAL ONLY

MEMBER  
**AIBD**  
AMERICAN INSTITUTE OF  
BUILDING DESIGN

STAY, LLC  
2405 KENDRICK DRIVE  
CHARLOTTE, NC 28214  
www.stayplans.com

**FOUNDATION NOTES:**  
ALL FIRST FLOOR FRAMING IS 2X10 @ 16" OC UNO  
DI = DOUBLE ENDIT  
PROVIDE SOLID WOOD UNDER ALL POINT LOADS UNO  
ALL HAND DRAWN DIMENSIONS ARE SCALED AND ROUNDED TO THE NEAREST 1/8 FOOT UNO  
PROVIDE TERMITE TREATMENT AND 6 MIL POLY VAPOR BARRIER UNDER ALL SLABS UNO

**FOUNDATION KEY**  
 8"X16" CMU PIER OVER 24"X36"X10" COOL. FIB.

CLIENT NO.	2023-48
DATE	8.4.23
REVISIONS	
DRAWN BY	
CHECKED BY	
DATE	
SCALE	

SHEET NO.  
**S1**



Sustainable Engineering & Efficient Design,  
 PLLC  
 3446 Greenleaf Avenue  
 Charlotte, NC 28208  
 SEAL FOR STRUCTURAL ONLY

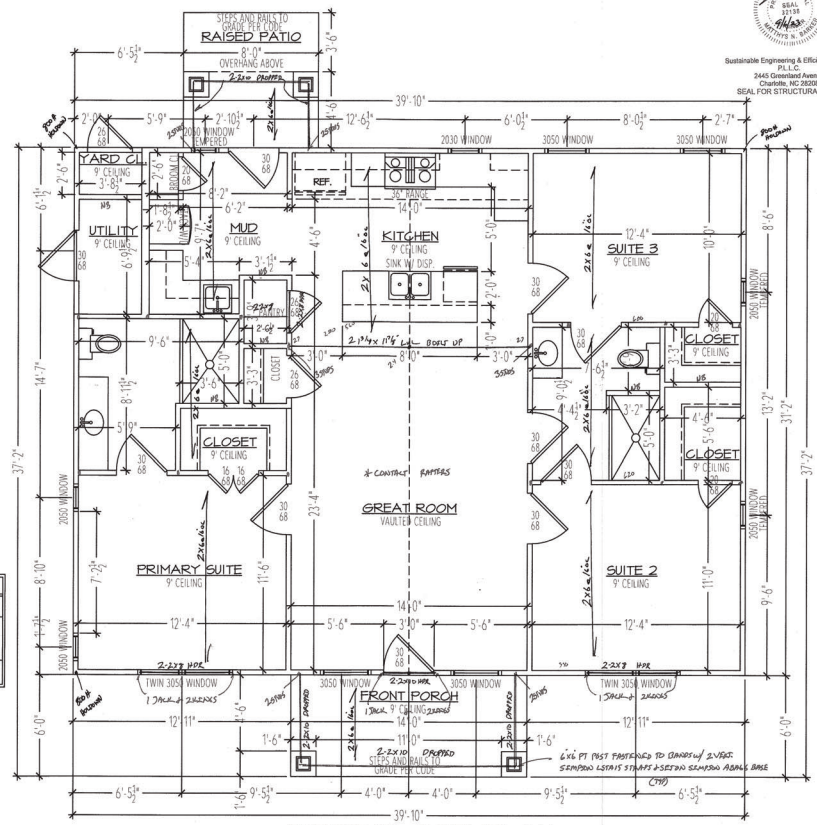
MEMBER  
**AIBD**  
 AMERICAN INSTITUTE OF  
 BUILDING DESIGN



**STAY LLC**  
 COBLE RESIDENCE  
 2405 KENDRICK DRIVE  
 CHARLOTTE, NC 28214

DATE: 8.4.23

SHEET NO.  
**S2**

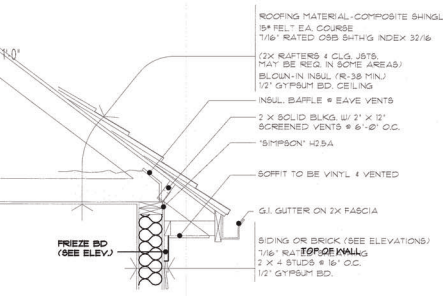
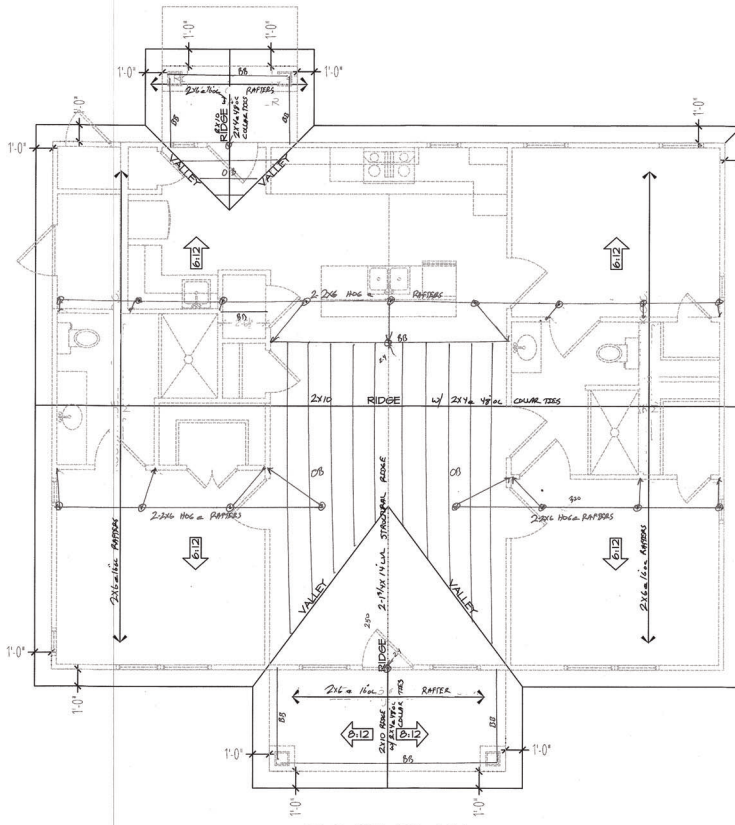


**FIRST FLOOR NOTES:**  
 ALL ATTIC FRAMING IS 2X8 @ 16" OC UNO  
 ALL FIRST FLOOR HEADERS ARE 2X8 WITH 1 JACK & 1 KING STUD UNO  
 PROVIDE 2 STUDS UNDER ALL ROOF BRACES UNO  
 FURNISH CONTACT FASTENERS FOR INSULATION UNO  
 FASTEN CONTACT FASTENERS TO EXTERIOR WALL TOP PLATES WITH SIMPSON  
 HQ SA HURRICANE STRAPS @ 12" OC  
 WALL BRACING PROVIDED BY CONT. 1" OSB SHEATHING FASTENED WITH 8D NAILS  
 @ 6" OC ON EDGE & 12" OC IN THE FIELD TO MEET & EXCEED THE INTENT OF  
 SECTION 602.30, ENGINEERED DESIGN UNO  
 NB = NON BEARING

SQUARE FOOTAGE BLOCK	
FIRST FLOOR	1,198 SQ. FT.
TOTAL HEATED	1,198 SQ. FT.
FRONT PORCH	84 SQ. FT.
UTILITY/YARD	40 SQ. FT.
COVERED PATIO	38 SQ. FT.
TOTAL UNDER ROOF	1,358 SQ. FT.
PATIO	28 SQ. FT.

**FIRST FLOOR PLAN**  
 SCALE: NTS





- ROOFING MATERIAL - COMPOSITE SHINGLES  
 3/4" FELT E.A. COURSE  
 1/4" RATED OSB (WITH INDEX 32/4)  
 2x RAFTERS 4 CLG. JOBS  
 MAY BE REG. IN SOME AREAS)  
 BLOW-IN INSUL (R-38 MIN.)  
 1/2" GYPSUM BD. CEILING  
 INSUL. BATTLE # EAVE VENTS  
 2 X SOLID BLKS. 1/2" X 1/2"  
 SCREENED VENTS # 6" - 2" O.C.  
 'SIMPSON' H2.B.A.  
 SOFFIT TO BE VINYL 4 VENTED  
 G.I. GUTTER ON 2x FASCIA  
 SIDING OR BRICK (SEE ELEVATIONS)  
 7/16" RAFTERS WALLS  
 2 X 4 STUDS # 16" O.C.  
 1/2" GYPSUM BD.

- ROOF NOTES:  
 ALL RAFTERS ARE 2X6 @ 36" O.C. UNO  
 ALL RIDGES ARE 2X12 WITH 2X4 @ 48" O.C. COLLAR TIES UNO  
 ALL HIPS AND VALLEYS ARE 2X12 UNO  
 BB = BEAM BELOW  
 RB = RAFTERS BEAR  
 OB = OVERBUILD



Sustainable Engineering & Efficient Designs,  
 P.L.L.C.  
 3445 Ophidian Avenue  
 Charlotte, NC 28206  
 SEAL FOR STRUCTURAL ONLY

MEMBER  
**AIBD**  
 AMERICAN INSTITUTE OF  
 BUILDING DESIGN



STAY LLC  
 CORLE RESIDENCE  
 2405 KENDRICK DRIVE  
 CHARLOTTE, NC 28214

STAY LLC  
 CORLE RESIDENCE  
 2405 KENDRICK DRIVE  
 CHARLOTTE, NC 28214

COMD NO.	2023-48
DATE	8.4.23
REVISIONS	
CREATED BY	CAD

DATE: 8.4.23  
 SHEET NO.  
**S3**